

# Analysis of Some Hydrological Parameters of Ghiladhari River, Assam, India

## Abstract

River bank erosion has become the major threat and disaster for the river bank dwellers due to continuing change of hydrological aspects of any river. The dynamism of hydrological aspects within the river leads to lateral erosion in the river bank which ultimately enhances the shifting of river channels. Various studies report increasing bank migration rates with either bank full discharge or stream power (Hooke 1979; Walker and Rutherford 1999; Alber and Piégay 2017). Now a days, the population pressure on the limited land resources has resulted on the higher settlement density not only in the hazardous riverbank areas but also some of the hazard prone areas. An understanding of the dynamic trend in terms of frequency and magnitude of hydrological variables within a river has become very essential for the implementation of hydrological hazard mitigation strategies. Through this paper an attempt has been made to analyze some of the hydrological aspects of Ghiladhari river to establish the relationship between basin parameters and hydrological parameters.

**Keywords:** Hydrological, Hazardous, Magnitude, Mitigation Andincessant. **Introduction**

The dynamism of hydrological aspects within the river leads to lateral erosion in the river bank which ultimately enhances the shifting of river channels. Various studies report that increasing bank migration rates with either bank full discharge or stream power (Hooke 1979; Walker and Rutherford 1999; Alber and Piégay 2017). The present study attempts to highlights the temporal change of stage & discharge magnitude as well as to estimate the future flood magnitude of this river.

### Study Area

The location of the study area belonged to the Sonitpur district of Assam, India. The Ghiladhari river is one of the important tributary of Brahmaputra River with its original source in the foot-hills of east Kameng district, Arunachal Pradesh. This river basin extends from 26°42'22" N to 26°58' N and 93°0'3" E to 98°8'24" E. having an approximate area of 150.66 Sq. Km. The Ghiladhari river have hilly origin and flow mostly in the plains of Sonitpur district of Assam (Fig. 1).

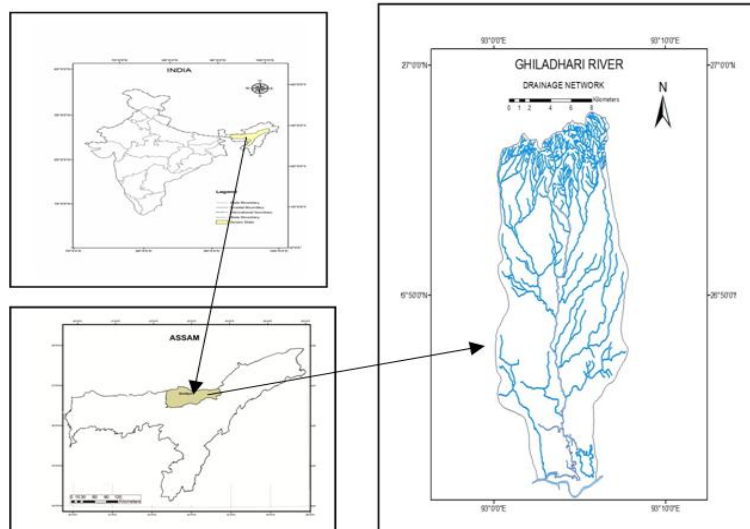
Fig 1: Location of The Study Area



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**Aim of the Study**

The main aim of this paper is to study some of the basic hydrological aspects of Ghiladhari River in Assam. The paper will establish the relationship between basin parameters and hydrological parameters in particular at micro level for understanding the fluvio-geomorphic processes in general.

**Database and Methodology**

The different sources of the secondary data relevant to the study include –

1. Survey of India (SOI) toposheets for the year 1971 (Scale-1:50,000) are used to prepare the base map.
2. The relevant hydraulic data are collected from Water resource Department, Government of Assam & IMD websites.
3. Arc GIS software and Microsoft office 2007 have been used to generate the required thematic maps and
4. The primary sources include the personal visit to the study area for the ground verification of the map.

**Objectives**

The main objectives of the study are-

1. To highlight the pattern of fluctuations of stages & discharges of Ghiladhari River,
2. To represent the geometric relationship between discharge & cross sectional area and discharge & velocity; and
3. To estimate different flood magnitudes of the river using flood frequency analysis method.

**Analysis of Hydrological Parameters**

The dynamism of hydrological aspects within the river leads to lateral erosion in the river bank

which ultimately enhances the shifting of river channels. Various studies report that increasing bank migration rates with either bank full discharge or stream power (Hooke 1979; Walker and Rutherford 1999; Alber and Piégay 2017). The present study attempts to highlight some of the hydrological aspects likethe temporal change of stage & discharge magnitude, hydraulic geometric relationsas well as the estimation of the future flood magnitude of this river.

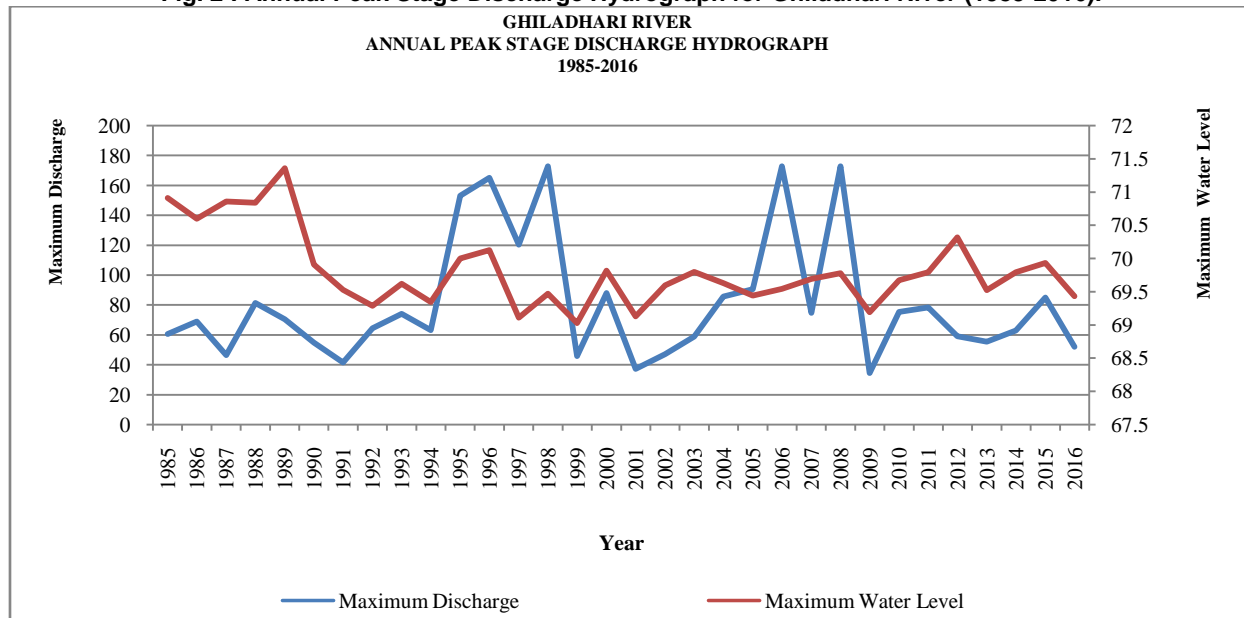
**Hydrograph Analysis**

The study of Annual Peak Stage Discharge Hydrograph, Annual Maximum and Minimum Discharge Hydrograph & Monthly Average Stage Discharge Hydrograph has been considered to be correlated for analyzing river characteristics associated with river bank erosion.All natural river, irrespective of their drainage basin properties and regime characteristics, experience high flows following heavy precipitation in their catchment areas(Leopold ,1964). The temporal fluctuation of Water Discharge & Stages for the Ghiladhari River is analyzed through

1. Annual Peak Stage Discharge Hydrograph (1985 to 2016).
2. Annual Maximum and Minimum Discharge Hydrograph (1985 - 2016).
3. *Annual Peak Stage Discharge Hydrograph (1985 to 2016)* :

The analysis of the annual peak stage discharge hydrograph of Ghiladhari river for the period of 32 years (1985 to 2016)represents the lowest peak discharge value of 37.13 m<sup>3</sup>/s (2001) and the highest one is 2172.82 m<sup>3</sup>/s (1998). On the other hand the lowest value of peak stage is found to be 69.10 min 2001 and the highest one is 71.36 m in 1998 (fig. 2).

**Fig. 2 : Annual Peak Stage Discharge Hydrograph for Ghiladhari River (1985-2016).**

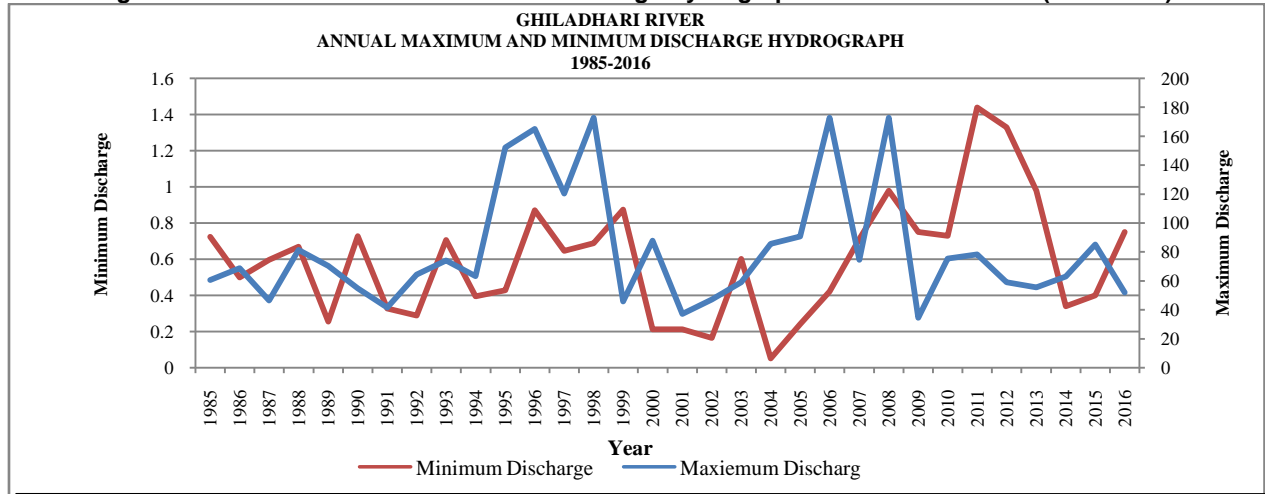


**Annual Maximum and Minimum Discharge Hydrograph (1985 - 2016):**

The analysis of the Annual Maximum and Minimum Discharge Hydrograph of Ghiladhari river (fig. 3) for the period of 32 years (1985 to

2016)represents highest discharge magnitude as 172.825m<sup>3</sup>/s (1998) and the minimum discharge magnitude as 0.34m<sup>3</sup>/s (2004).

Fig. 3 : Annual Maximum and Minimum Discharge Hydrograph for Ghiladhari River (1985-2016)



**Hydraulic Geometric Relation**

Hydraulic geometry was coined by Leopold and Maddock (1953) for the quantitative description of how river depth, width, velocity and related properties vary with changing discharge. In this paper analysis has been done to represent the relation between X sectional area & water discharge values and water discharge & velocity values. In the first case, the

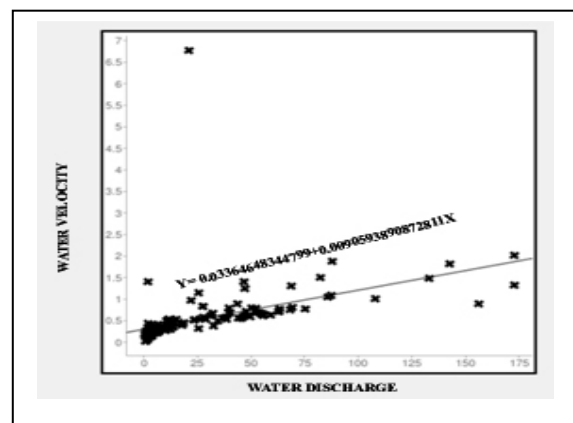
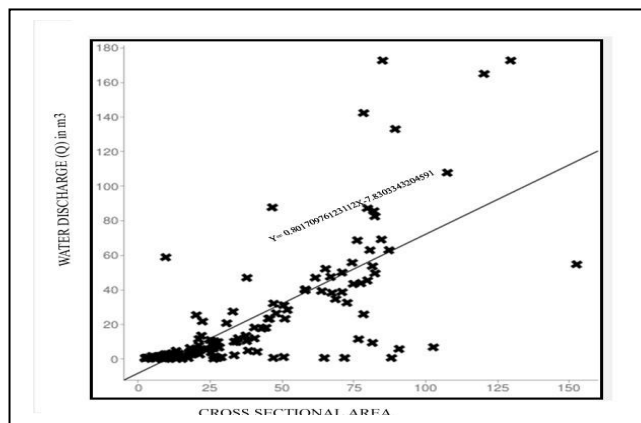
water discharge and cross sectional data for the period of ,12 years(1994-2016)has been collected and graphically represented through fig. 4. Here we found the value of 'r' (0.62632584) shows a positive relation between the water discharge and cross section area. The dependent variable water discharge plotted against the independent variable cross sectional area.

$$\begin{aligned} \sum \text{Log } X &= 202.80977 & \sum \text{Log } Y &= 113.531871 & \sum \text{Log } X &= 157.0291313 & \sum \text{Log } Y &= -55.9043231 \\ \sum \text{Log } X^2 &= 310.657882 & \sum \text{Log } Y^2 &= 187.682137 & \sum \text{Log } X^2 &= 220.830175 & \sum \text{Log } Y^2 &= 41.2154903 \\ \sum \text{Log } XY &= 190.939767 & r &= 0.62632584 & \sum \text{Log } XY &= 190.939767 & r &= 0.919830 \end{aligned}$$

(Source: regression analysis calculator, www.alcula.com)

Fig. 4 : Cross Sectional Area Vs Water Discharge of Ghiladhari River (1994-2016)

Fig. 5 :Water Discharge Vs Water Velocity of Ghiladhari River (1994-2016)



The second analysis representing the relation between water discharge and water velocity shows a highly positive correlation co-efficient value i. e. r = 0.919830 (fig. 5).

**Estimation of Flood Magnitudes**

The volume of water discharge is highly correlated with the bank erosion events in most of the rivers. In the monsoonal rivers, high magnitudes floods occur at an interval of several years to decades. Peak discharges are two to four times more than the mean monsoon discharges (Kale, 1998). The value of annual maximum flood from a given catchment area of the large numbers of the successive year constitutes a hydrological data series called the

annual series. The data are then arranged in descending order of magnitude and probability of each event is calculated by different methods of flood frequency analysis. The estimation of flood magnitude for Ghiladhari river has been done by using Semi-log plotting position method and Gumbels' Extreme value distribution method (fig. 5 & 6 and table 1,2 & 3).

**Findings**

The estimated flood magnitudes of Ghiladhari river (1985-2016) for the return period of 20 and 30 and 50 years using semi Log plotting position method are found as 128 m³/s, 141 m³/s and 151 m³/s.

Fig. 5  
Flood Frequency Analysis by Semi-log Plotting Position Method for Ghiladhari River, Sonitpur, Assam (1985-2016)

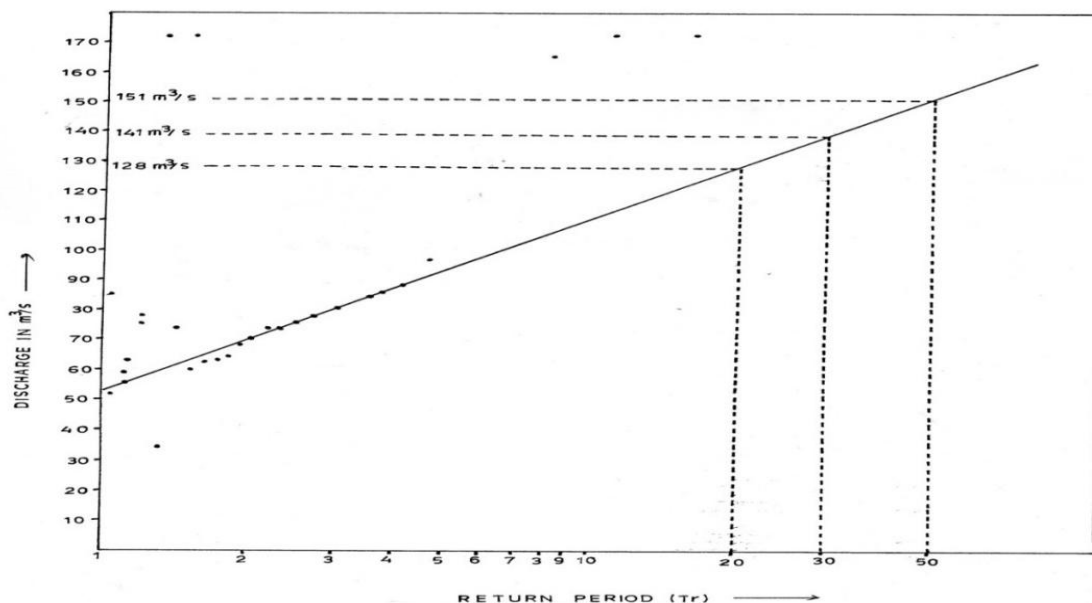


Table 1: Computation of the Return Period and Probability for flood magnitude of Ghiladhari River (1985-2016) by using WEIBULL'S SEMI LOG PLOTTING POSITION METHOD

| Year | Peak Flood | Peak Flood (Descending Order) | Rank | Probability $P = \frac{m}{n} + 1$ | Return Period $Tr = \frac{n+1}{m}$ | Year | Peak Flood | Peak Flood (Descending Order) | Rank | Probability $P = \frac{m}{n} + 1$ | Return Period $Tr = \frac{n+1}{m}$ |
|------|------------|-------------------------------|------|-----------------------------------|------------------------------------|------|------------|-------------------------------|------|-----------------------------------|------------------------------------|
| 1985 | 60.446     | 172.825                       | 1    | 0.0303                            | 33                                 | 2001 | 37.13      | 68.848                        | 17   | 0.5151                            | 1.941                              |
| 1986 | 68.848     | 172.79                        | 2    | 0.0606                            | 16.5                               | 2002 | 46.976     | 64.329                        | 18   | 0.5454                            | 1.833                              |
| 1987 | 46.475     | 172.79                        | 3    | 0.0909                            | 11                                 | 2003 | 58.935     | 63.14                         | 19   | 0.5757                            | 1.736                              |
| 1988 | 81.359     | 165.078                       | 4    | 0.1212                            | 8.25                               | 2004 | 85.67      | 63.01                         | 20   | 0.606                             | 1.65                               |
| 1989 | 70.328     | 152.131                       | 5    | 0.1515                            | 6.6                                | 2005 | 90.63      | 60.646                        | 21   | 0.6363                            | 1.571                              |
| 1990 | 54.841     | 120.257                       | 6    | 0.1818                            | 5.5                                | 2006 | 172.79     | 59.05                         | 22   | 0.6666                            | 1.5                                |
| 1991 | 41.472     | 90.63                         | 7    | 0.2121                            | 4.714                              | 2007 | 74.62      | 58.935                        | 23   | 0.6969                            | 1.434                              |
| 1992 | 64.329     | 87.888                        | 8    | 0.2424                            | 4.125                              | 2008 | 172.79     | 55.52                         | 24   | 0.7272                            | 1.375                              |
| 1993 | 74.329     | 85.67                         | 9    | 0.2727                            | 3.666                              | 2009 | 34.44      | 54.841                        | 25   | 0.7575                            | 1.32                               |
| 1994 | 63.14      | 85.09                         | 10   | 0.303                             | 3.3                                | 2010 | 75.41      | 52.04                         | 26   | 0.7878                            | 1.269                              |
| 1995 | 152.13     | 81.359                        | 11   | 0.3333                            | 3                                  | 2011 | 78.37      | 46.976                        | 27   | 0.8181                            | 1.222                              |
| 1996 | 165.07     | 78.37                         | 12   | 0.3636                            | 2.75                               | 2012 | 59.05      | 46.475                        | 28   | 0.8484                            | 1.178                              |
| 1997 | 120.25     | 75.41                         | 13   | 0.3939                            | 2.538                              | 2013 | 55.52      | 45.764                        | 29   | 0.8787                            | 1.137                              |
| 1998 | 172.82     | 74.62                         | 14   | 0.4242                            | 2.357                              | 2014 | 63.01      | 41.472                        | 30   | 0.909                             | 1.1                                |
| 1999 | 45.764     | 74.176                        | 15   | 0.4545                            | 2.2                                | 2015 | 85.09      | 37.13                         | 31   | 0.9393                            | 1.064                              |
| 2000 | 87.888     | 70.328                        | 16   | 0.4848                            | 2.062                              | 2016 | 52.04      | 34.44                         | 32   | 0.9696                            | 1.031                              |

Data Source : Water Resource Department, Govt. of Assam, India.

This extreme value distribution was introduced by Gumbel (1994) and it is commonly known as Gumbel's distribution. It is one of the most widely used probability distribution functions for extreme values in hydrologic and meteorological

studies for prediction of flood peaks, maximum rainfall, maximum wind speed etc. This method has been applied for the present study for the estimation of flood magnitude of Ghiladhari river for the period of 32 years (fig. 6 and table 2 & 3).

Table 2. Computation of the Return Period and Probability for flood magnitude of Ghiladhari river (1998- 2016) by using "GUMBELL'S EXTREME VALUE DISTRIBUTION METHOD."

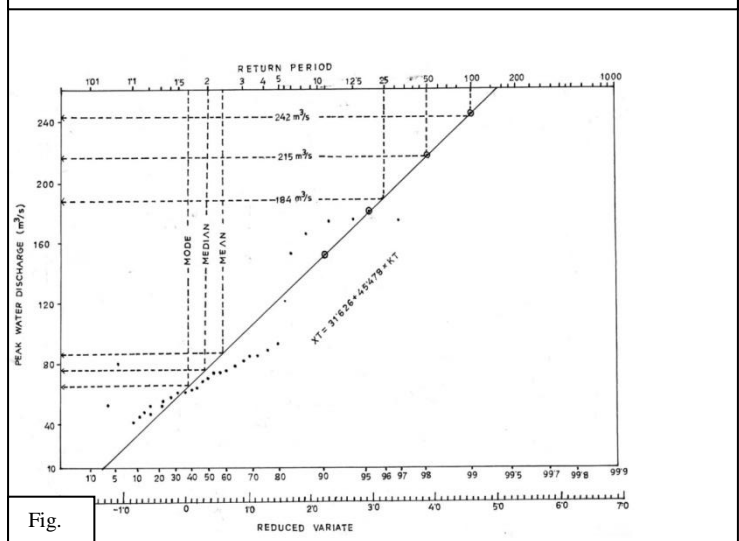
| Year | Peak Flood (m <sup>3</sup> /s) | Peak Flood in Descending Order | Order (m) | (x-x) <sup>2</sup> | Return Period $T_r = \frac{n+1}{m}$ | Reduced Variate $Y_T = -[\ln \ln \frac{T_r}{T_r-1}]$ | Year | Peak Flood (m <sup>3</sup> /s) | Peak Flood in Descending Order | Order (m) | (x-x) <sup>2</sup> | Return Period $T_r = \frac{n+1}{m}$ | Reduced Variate $Y_T = -[\ln \ln \frac{T_r}{T_r-1}]$ |
|------|--------------------------------|--------------------------------|-----------|--------------------|-------------------------------------|--|------|--------------------------------|--------------------------------|-----------|--------------------|-------------------------------------|--|
| 1985 | 60.446                         | 172.825                        | 1         | 8317.26            | 33                                  | 3.457  | 2001 | 37.13                          | 68.848                         | 17        | 163.27             | 1.941                               | 0.323  |
| 1986 | 68.848                         | 172.79                         | 2         | 8310.87            | 16.5                                | 2.78   | 2002 | 46.976                         | 64.329                         | 18        | 299.18             | 1.833                               | 0.237  |
| 1987 | 46.475                         | 172.79                         | 3         | 8310.87            | 11                                  | 2.35   | 2003 | 58.935                         | 63.14                          | 19        | 341.73             | 1.736                               | 0.153  |
| 1988 | 81.359                         | 165.078                        | 4         | 6964.24            | 8.25                                | 2.052  | 2004 | 85.67                          | 63.01                          | 20        | 346.55             | 1.65                                | 0.071  |
| 1989 | 70.328                         | 152.131                        | 5         | 4970.95            | 6.6                                 | 1.808  | 2005 | 90.63                          | 60.646                         | 21        | 440.16             | 1.571                               | -0.011   |
| 1990 | 54.841                         | 120.257                        | 6         | 1492.35            | 5.5                                 | 1.606  | 2006 | 172.79                         | 59.05                          | 22        | 509.67             | 1.5                                 | -0.094   |
| 1991 | 41.472                         | 90.63                          | 7         | 81.07              | 4.714                               | 1.434  | 2007 | 74.62                          | 58.935                         | 23        | 514.88             | 1.434                               | -0.178   |
| 1992 | 64.329                         | 87.888                         | 8         | 39.21              | 4.125                               | 1.281  | 2008 | 172.79                         | 55.52                          | 24        | 681.52             | 1.375                               | -0.261   |
| 1993 | 74.329                         | 85.67                          | 9         | 16.35              | 3.666                               | 1.144  | 2009 | 34.44                          | 54.841                         | 25        | 717.44             | 1.32                                | -0.348   |
| 1994 | 63.14                          | 85.09                          | 10        | 11.99              | 3.3                                 | 1.02   | 2010 | 75.41                          | 52.04                          | 26        | 875.33             | 1.269                               | -0.439   |
| 1995 | 152.131                        | 81.359                         | 11        | 0.07               | 3                                   | 0.902  | 2011 | 78.37                          | 46.976                         | 27        | 1200.62            | 1.222                               | -0.533   |
| 1996 | 165.078                        | 78.37                          | 12        | 10.60              | 2.75                                | 0.0794   | 2012 | 59.05                          | 46.475                         | 28        | 1235.59            | 1.178                               | -0.636   |
| 1997 | 120.257                        | 75.41                          | 13        | 38.64              | 2.538                               | 0.691  | 2013 | 55.52                          | 45.764                         | 29        | 1286.08            | 1.137                               | -0.749   |
| 1998 | 172.825                        | 74.62                          | 14        | 49.08              | 2.357                               | 0.594  | 2014 | 63.01                          | 41.472                         | 30        | 1612.34            | 1.1                                 | -0.874   |
| 1999 | 45.764                         | 74.176                         | 15        | 55.51              | 2.2                                 | 3.08   | 2015 | 85.09                          | 37.13                          | 31        | 1979.89            | 1.064                               | -1.033   |
| 2000 | 87.888                         | 70.328                         | 16        | 127.64             | 2.062                               | 0.411  | 2016 | 52.04                          | 34.44                          | 32        | 2226.52            | 1.031                               | -1.253   |

Table 3 : Computation of flood magnitude for different Return Periods by Gumbel's Extreme Value Distribution Method for Ghiladhari River (1985-2016).

| Return Period (T <sub>r</sub> ) | X     | S <sub>x</sub> | Frequency Factor $K_T = Y_T - \frac{Y_n}{S_n}$ | X <sub>T</sub> = X + K <sub>T</sub> S <sub>x</sub> |
|---------------------------------|-------|----------------|--|--|
| 10                              | 81.62 | 45.47          | 1.5306   | 151.2346   |
| 20                              |       |                | 2.1834   | 180.922  |
| 50                              |       |                | 3.0232   | 219.1150   |
| 100                             |       |                | 3.4049   | 236.4740   |
| 200                             |       |                | 3.6380   | 247.0749   |

The estimated flood magnitude for the return period of 10, 20, 50 & 100 years are 151.2346 m<sup>3</sup>/s, 180.922 m<sup>3</sup>/s, 219.1150 m<sup>3</sup>/s and 236.4740 m<sup>3</sup>/s.

FLOOD FREQUENCY ANALYSIS BY GUMBEL'S EXTREME VALUE DISTRIBUTION METHOD GHILADHARI RIVER, SONITPUR, ASSAM 1985-2016



Conclusion

The present paper tries its best to outline the frequency variation of stage & discharge of Ghiladhari River, the representative hydraulic geometrical

relationship in between cross sectional area, velocity and water discharge and flood magnitude estimation. The main objectives of this paper are fulfilled in the foregoing representations and analysis of relevant

data as indicated in different tables and figures. The precise representation of different aspects of the topic as mentioned in different sections of the paper will be useful to the environmentalists, planners as well as research workers.

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